GENERAL NOTES:

DESIGN CRITERIA:

A. CODES AND STANDARDS:

2015 INTERNATIONAL BUILDING CODE (IBC)

2015 INTERNATIONAL EXISTING BUILDING CODE (IEBC) TO COMPLY WITH CHAPTER 301.3.1 "PRESCRIPTIVE COMPLIANCE METHOD"

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

   ALL WF (U.N.O.): A992 GRADE 50 (Fy=50 K.S.I.)
   ALL ANGLE, BASE PLATES, CONN. PLATES (U.N.O.): A36 (Fy=36 K.S.I.)
   STRUCTURAL PIPE: A53 (Fy=35 K.S.I.)

2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE A.I.S.C. CODE OF STANDARD PRACTICE (LATEST EDITION), EXCEPT AS MODIFIED IN THESE NOTES.

3. ALL BOLTS ARE ASTM A325N (U.N.O.)

4. ALL WELDING RODS SHALL BE E70 ELECTRODES

5. ELECTRODE SHALL MEET CHARPY V-NOTCH TOUGHNESS REQUIREMENTS WHERE REQUIRED FOR BASE METAL.

6. WELDS SHALL NOT BE LESS THAN THE GREATEST OF:

   A. THOSE INDICATED ON CONTRACT DOCUMENTS

   B. THE MINIMUM REQUIRED BY AWS D1.1 AND AISC SPECIFICATIONS

   C. FILLET WELDS: 3/16"

   D. GROOVE WELDS: COMPLETE JOINT PENETRATION

7. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND AWS. NON-PREQUALIFIED WELDS SHALL BE QUALIFIED AT CONTRACTOR'S EXPENSE PRIOR TO FABRICATION.

8. WELDS INDICATED ARE CONTINUOUS.

9. BEAMS, CONNECTION DESIGN FORCES – GREATER OF:

   A. 55% OF TOTAL ALLOWABLE UNIFORM LOAD CAPACITY FROM A.I.S.C 9TH EDITION TABLES FOR ALLOWABLE LOADS ON BEAMS, Wc/L.

   B. REACTIONS SHOWN ON DRAWINGS.

   C. 10 KIPS.

10. THE MINIMUM PLATE THICKNESS SHALL BE 3/8".

11. BOLTED CONNECTIONS:

   A. MINIMUM BOLT DIAMETER = 3/4" U.N.O.

   B. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE USING A325N OR A490N BOLTS. OVERSIZED HOLES AND LONG-SLOTTED HOLES ARE NOT ALLOWED UNLESS SHOWN ON THE DRAWINGS.

   C. A307 BOLTS MAY BE USED WHERE INDICATED ON THE DRAWINGS.

12. WELDED CONNECTIONS:

   A. WELDS ARE CONTINUOUS UNLESS NOTED.

   B. ALL FILLET WELDS: A.I.S.C. MINIMUM BUT NOT LESS THAN 3/16" UNLESS NOTED OTHERWISE.

   C. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE" (LATEST EDITION) PUBLISHED BY THE AMERICAN WELDING SOCIETY. ELECTRODES FOR WELDING SHALL COMPLY WITH THE REQUIREMENTS OF A.W.S. D1.1 & D1.1M.

   D. ALL GROOVE WELDS SHALL BE COMPLETE PENETRATION UNLESS NOTED OTHERWISE.

13. SPLICING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE ENGINEER.

14. NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE AND HOLES, SLOTS, CUTS, ETC., ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS THEY ARE DETAILED ON THE APPROVED SHOP DRAWINGS.

15. NO FINAL BOLTING OR WELDING SHALL BE MADE UNTIL AS MUCH OF THE STRUCTURE AS WILL BE STIFFENED THEREBY HAS BEEN PROPERLY ALIGNED.

16. FABRICATE ALL BEAMS WITH THE MILL CAMBER UP.

17. ALL NEW STEEL SHALL RECEIVE A STANDARD 1.5 MIL COAT OF SHOP PRIMER AND A FINISHED COAT OF 2.0 MILS OSHA SAFETY YELLOW PAINT UNLESS NOTED OTHERWISE.

18. ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F1554 GRADE 36 (MIN.). ANCHOR BOLTS, NUTS, & MISCELLANEOUS HARDWARE SHALL BE GALVANIZED PER ASTM A153.

19. CAST-IN ANCHOR BOLTS SHALL HAVE A HEAVY HEX NUT PER ASTM A563 AT THE BOTTOM. THE NUT SHALL BE PREVENTED FROM SPINNING OFF DURING INSTALLATION

CONCRETE:

1. ALL CONCRETE SHALL HAVE A 28 DAY MINIMUM COMPRESSIVE STRENGTH OF 5000 P.S.I. UNLESS NOTED OTHERWISE, DENSITY OF 145 P.C.F., AND MAXIMUM WATER TO CEMENT RATIO OF 0.40. CONCRETE MIX SHALL CONFORM WITH THE REQUIREMENTS SET FORTH IN ACI 301.

2. THE MAXIMUM WATER-SOLUBLE CHLORIDE ION CONTENT IN CONCRETE ACCORDING TO PERCENT BY WEIGHT OF CEMENT SHALL BE 0.15.

3. AIR CONTENT SHALL CONFORM TO THE TABLE BELOW:

NOMINAL MAXIMUM AGGREGATE SIZE, IN.	TARGET AIR CONTENT, PERCENT
3/8	7.5
1/2	7.0
3/4	6.0
1	6.0
1-1/2	5.5
2	5.0
3	4.5

4. SUBMIT CONCRETE MIX DESIGNS FOR REVIEW.

5. REINFORCING SHALL CONFORM TO A.S.T.M. A615, GR. 60, INCLUDING TIES AND STIRRUPS.

6. WELDED WIRE FABRIC SHALL CONFORM TO A.S.T.M. A185.

7. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED, IN ACCORDANCE WITH A.C.I. DETAILING MANUAL (LATEST EDITION) (SP-66).

8. ALL REINFORCING SHALL BE SUPPORTED IN FORMS, SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER, IN ACCORDANCE WITH C.R.S.I. "REINFORCING BAR DETAILING" (LATEST EDITION).

9. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:

   A. UNFORMED SURFACE IN CONTACT WITH THE GROUND: 3 IN.

   B. FORMED SURFACES EXPOSED TO EARTH OR WEATHER: 1 1/2 IN. FOR #5 BAR OR SMALLER
      2 IN. FOR #6 BAR OR LARGER

   C. FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:

      1. WALLS, SLABS: 3/4 IN.

9. ALL CONSTRUCTION JOINTS SHOWN ON DRAWINGS SHALL BE INCORPORATED INTO THE STRUCTURE, UNLESS THEIR ELIMINATION IS APPROVED BY THE ENGINEER. ADDITIONAL CONSTRUCTION JOINTS, REQUIRED TO FACILITATE CONSTRUCTION, SHALL BE LOCATED AT POINTS OF MINIMUM SHEAR AND SHALL BE DETAILED ON SHOP DRAWINGS. REINFORCEMENT SHALL PASS CONTINUOUSLY THROUGH THE JOINT.

10. ALL ABUTTING CONCRETE MEMBERS SHALL BE DOWELED TOGETHER, UNLESS POURED MONOLITHICALLY. DOWELS SHALL BE EQUAL IN SIZE AND SPACING TO THE REINFORCING IN THE ADJACENT MEMBER.

11. PROVIDE 3/4" CHAMFERS AT ALL EDGES THAT ARE EXPOSED TO VIEW IN THE FINISHED STRUCTURE.

12. BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" CONCRETE.

13. PROVIDE STANDARD HOOKS ON BARS TERMINATING AT A CONCRETE FACE UNLESS NOTED (E.G.: EDGES OF OPENINGS, SLAB EDGES, EXPANSION JOINTS, ENDS OF BEAMS, AND AT: TOP, BOTTOM AND ENDS OF WALLS, ETC.).

MISCELLANEOUS:

1. NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

2. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

3. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.

4. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.

5. DO NOT SCALE THESE DRAWINGS, FOLLOW DIMENSIONS.

6. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY FOR SUCH DEVIATION BY THE ENGINEER'S APPROVAL OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS HE HAS SPECIFICALLY INFORMED THE ENGINEER OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ENGINEER HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.

7. ALL THINGS WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS, OR AMBIGUITIES, IN THE PLANS AND SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. PLANS AND/OR SPECIFICATIONS WILL BE CORRECTED, OR WRITTEN INTERPRETATION OF THE ALLEGED DEFICIENCY, OMISSION, CONTRADICTION OR AMBIGUITY WILL BE MADE BY THE ENGINEER BEFORE THE AFFECTED WORK PROCEEDS.

8. CHECK ALL DIMENSIONS AGAINST REQUIREMENTS OF OTHER CONTRACT DOCUMENTS. FIELD VERIFY DIMENSIONS RELATING TO EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS OR STARTING FABRICATION.

9. NO MODIFICATION, ALTERATION OR REPAIR SHALL BE MADE WITHOUT PRIOR REVIEW BY THE STRUCTURAL ENGINEER. SUBMIT DETAILS AND CALCULATIONS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN STATE WHERE PROJECT IS LOCATED AND EMPLOYED BY THE CONTRACTOR.

SPECIAL INSPECTION:

THE FOLLOWING STRUCTURAL ELEMENTS OF CONSTRUCTION SHALL REQUIRE SPECIAL INSPECTION PER IBC 2015 CHAPTER 17. THE SPECIAL INSPECTIONS LISTED BELOW WILL BE PROVIDED AND PAID FOR BY THE COUNTY.

TYPE OF CONSTRUCTION	IBC SECTION	IBC TABLE
COMPONENT FABRICATION	1704.2.5	–
STEEL CONSTRUCTION	1705.2	–
CONCRETE	1705.3	1705.3
POST INSTALLED ANCHORS	–	1705.3

NOTES:

A. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A CITY INSPECTOR. WORK WHICH IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE CITY INSPECTOR IS SUBJECT TO REMOVAL OR EXPOSURE.

B. THE SPECIAL INSPECTORS MUST BE CERTIFIED BY THE CITY TO PERFORM THE TYPES OF INSPECTION SPECIFIED.

C. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.

D. SUBMIT WRITTEN REPORTS WITHIN TWO DAYS OF TESTING/INSPECTION TO ENGINEER OF RECORD.

POST INSTALLED ANCHORS:

1. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY SIMPSON OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.

2. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS/RECOMMENDATIONS.

3. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

STANDARD STRUCTURAL ABBREVIATIONS

ABOVE FINISHED FLOOR	A.F.F.	EXISTING	(E)	PLATE	℄
ADDENDUM	ADD.	EXPANSION JOINT	E.J.	PLUMBING	PLBG
ADDITIONAL	ADDL.	EXTERIOR	EXT.	PLYWOOD	PWD
AGGREGATE	AGG.	FAR SIDE	F.S.	PRECAST CONCRETE	P.C.
ALUMINUM	ALUM.	FIELD VERIFY	F.V.	POST TENSION	P.T.
ANCHOR BOLT	A.B.	FINISH	FIN.	PREMOLDED JOINT FILLER	P.J.F.
AND	&	FINISH FLOOR	F.F.	RADIUS	RAD.
ANGLE	∠	FLOOR	FLR.	RECTANGULAR	RECT.
ARCHITECTURAL	ARCH.	FOOTING	FTG.	REFERENCE	REF.
AT	@	FOUNDATION	FND.	REINFORCED CONCRETE	R.C.
BASEMENT	BSMT.	FRAMING	FRMG.	REINFORCING	REINF.
BASE PLATE	B. ℄	GALVANIZED	GALV.	REMAINDER	REM.
BEAM	BM.	GAUGE	GA.	REQUIRED	REQ'D
BEAMS	BMS.	GENERAL CONTRACTOR	G.C.	REVISION	REV.
BEARING	BRG.	GIRDER	GIR.	RISER	RSR.
BETWEEN	BTWN.	GRADE	GR.	ROOF	RF.
BLOCK	BLK.	GROUT	GT.	ROOM	RM
BOARD	BD.	GYPSUM	GYP.	ROUGH OPENING	R.O.
BOTTOM	BOT.	HEIGHT	HT.	SCHEDULE	SCHD.
BOTTOM FACE	B./FACE	HIGH POINT	H.P.	SECTION	SECT.
BOTTOM OF METAL DECK	B.M.D.	HOLLOW CORE	H.C.	SHEAR CONNECTORS	S.C.
BOTTOM OF FOOTING	B.O.F.	HORIZONTAL	HORIZ.	SHEAR WALL	SH. W.
BOTTOM OF WALL	B.O.W.	HORIZONTAL EACH FACE	H.E.F.	SHORT WAY	S.W.
BRICK	BRK.	INSIDE FACE	I.F.	SIMILAR	SIM.
BRIDGING	BRDG.	INTERIOR	INT.	SLAB	SL.
BUILDING	BLDG.	JOINT	JT.or.JNT.	SLAB ON GRADE	S.O.G.
CANTILEVER	CANTL.	JOIST	JST.	SPACE, SPACING, SPACES	SPA.
CAST IN PLACE	C.I.P.	KIP = 1000 lbs.	K	SPECIFICATIONS	SPECS.
CATEGORY	CAT.	KNOCKOUT	KO	SQUARE	SQ.
CEILING	CLG.	POUND	lb.	STANDARD	STD.
CENTER LINE	℄	LIGHT WEIGHT	L.T. WT.	STEEL	STL.
CENTER TO CENTER	C/C	LINTEL	LTL.	STRUCTURE OR STRUCTURAL	STRUC
CENTERED	CTR'D.	LIVE LOAD	L.L.	SUPPORT	SUPPT.
CLEAR	CLR.	LONG	LG.	SYMMETRICAL	SYM.
CONCRETE	CONC.	LONGITUDINAL	LONG.	THICKNESS	THK.
COLUMN	COL.	LONG LEG HORIZONTAL	L.L.H.	THREAD	THRD.
COMPLETE PENETRATION	C.P.	LONG LEG VERTICAL	L.L.V.	TO BE DETERMINED	T.B.D.
CONNECTION	CONN.	LONG WAY	L.W.	TOP & BOTTOM	T&B
CONSTRUCTION	CONST.	LOW POINT	L.P.	TOP FACE	T./FACE
CONSTRUCTION JOINT	C.J.	MANUFACTURER	MFGR.	TOP OF FOOTING	T.O.F.
CONTINUOUS	CONT.	MARK	MK.	TOP OF FOUNDATION	T.O.FND
CONTRACTOR	CONTR.	MASONRY	MAS.	TOP OF CONCRETE	T.O.C.
CRACK CONTROL JOINT	CNTR. J	MASONRY OPENING	M.O.	TOP OF JOIST	T.O.J.
DEAD LOAD	D.L.	MATERIAL	MAT'L	TOP OF LEDGE	T.O.L.
DECK	DK.	MAXIMUM	MAX.	TOP OF PILASTER	T.O.P.
DETAIL	DTL.	MECHANICAL	MECH	TOP OF SLAB	T.O.SL.
DIAGONAL	DIAG.	METAL	MTL.	TOP OF STRUCTURAL STEEL	T.O.S
DIAMETER	ø	MINIMUM	MIN.	TOP OF WALL	T.O.W.
DIMENSION	DIM.	MISCELLANEOUS	MISC.	TREAD	TRD.
DOUBLE	DBL.	MOMENT CONNECTION	M.C.	TYPICAL	TYP.
DOWELS	DWLS.	NEAR SIDE	N.S.	UNLESS NOTED OTHERWISE	U.N.O.
DOWN	DN.	NEW	(N)	VAPOR BARRIER	V.B.
DRAWINGS	DWGS.	NOMINAL	NOM.	VERTICAL	VERT
EACH	EA.	NOT IN CONTRACT	N.I.C.	VERTICAL EACH FACE	V.E.F.
EACH FACE	E.F.	NOT TO SCALE	N.T.S.	WIND BRACE	W.B.
EACH WAY	E.W.	ON CENTER	O.C.	WELDED WIRE FABRIC	W.W.F.
ELECTRICAL	ELECT.	OPENING	OPNG	WITH	W/
ELEVATION	EL.	OPPOSITE	OPP.	WITHOUT	W/O
ELEVATOR	ELEV.	OPPOSITE HAND	O.H.	WATER STOP	W.S.
EMBEDED	EMBE'D.	OUTSIDE FACE	O.F.	WEIGHT	WT.
EQUAL	EQ.	PANEL	PNL.	WORK POINT	W.P.
EQUIPMENT	EQUIP.	PENTHOUSE	P.H.		

ISSUED FOR CONSTRUCTION

NOTE: CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL EXISTING & PROPOSED DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

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LICENSE #: E-2006031671
EXPIRATION DATE: 12/31/26

CORPORATION
NAME: Structures, Inc.
DISCIPLINE: Engineering
CERT. OF AUTHORITY #: E-2006035000

STATE OF MISSOURI
ROY EDWARD JENNINGS III
NUMBER E-2006031671
PROFESSIONAL ENGINEER
9/3/2026

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ST. LOUIS COUNTY

MAINTENANCE FACILITY
DISTRICT 3 REPAIRS

CLAYTON, MO

REVISIONS

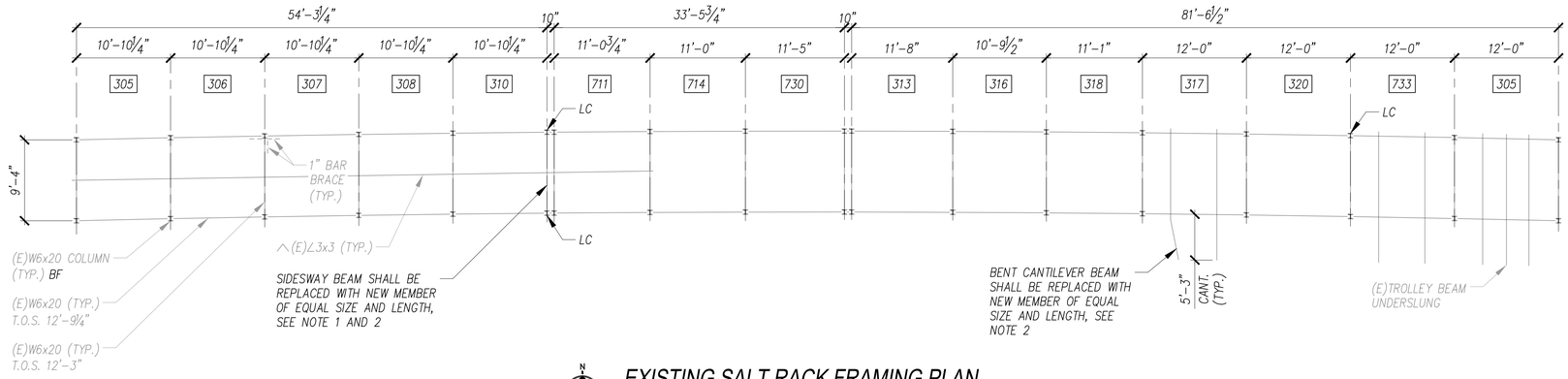
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SHEET INFORMATION

Date:	9/3/2026
Scale:	AS SHOWN
Drawn By:	MPV
Checked By:	REJ
Job Number:	P21-4431-02

Sheet No.
S1.0

TICK MARKS ON THE BINDING MARGIN AND UPPER MARGIN ARE @ 1" CENTERS. IF THE TICK MARKS ARE NOT @ 1" CENTERS, THIS SHEET HAS BEEN REPRODUCED AT A SCALE OTHER THAN 1:1
PRINT TO SCALE SHEET SIZE: 24x36 9/3/2026 9:44 AM

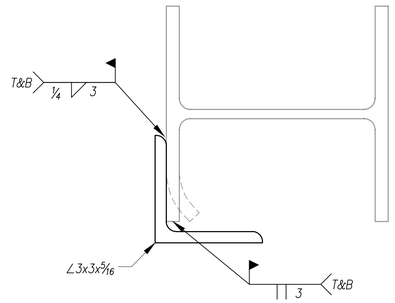


EXISTING SALT RACK FRAMING PLAN

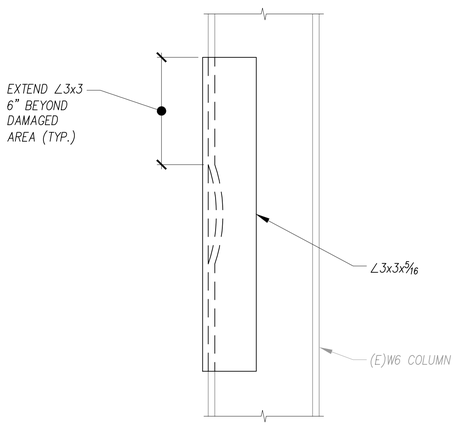
⅛" = 1'-0"

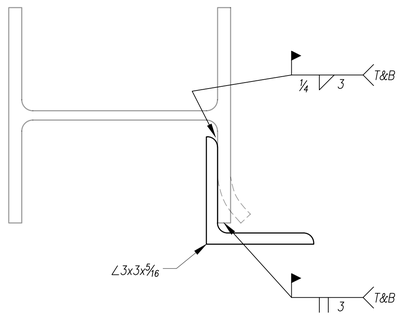
BF = BENT FLANGE
LC = LEANING COLUMN

NOTES:
1. FOR NEW BEAM, NEW KICKERS SHALL BE MADE AND ATTACHED IN KIND. FOR EXISTING BEAMS, EXISTING KICKERS SHALL BE REATTACHED TO THE NEW COLUMNS.
2. FOR NEW BEAMS, INSTALL ⅜" BEARING STIFFENERS AT SUPPORTS IN LINE WITH COLUMN FLANGES OR BEAM WEB.



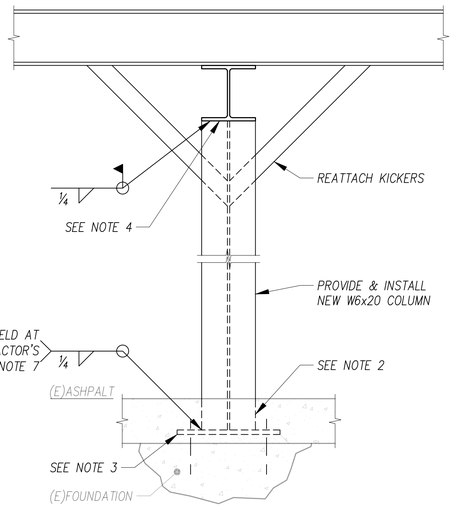
IF FLANGE IS BENT INWARD





IF FLANGE IS BENT OUTWARD

DAMAGED SALT RACK COLUMN REPAIR



REPLACE LEANING COLUMNS

SCALE: 1½"=1'-0"

NOTES:
1. TEMPORARILY SHORE EXISTING SALT RACK FRAMING.
2. EXCAVATE EXISTING ASPHALT AROUND BASE OF COLUMN, INCLUDING ENOUGH SPACE FOR COLUMN REMOVAL AND INSTALLATION. REPAIR ASPHALT FOLLOWING INSTALLATION OF NEW COLUMN.
3. FIELD VERIFY EXISTING COLUMN BASE CONSTRUCTION. FABRICATE REPLACEMENT COLUMN BASE TO MATCH EXISTING.
4. BURN OUT AND GRIND SMOOTH EXISTING WELDS THAT ATTACH COLUMN TO BEAM ABOVE.
5. RE-USE EXISTING ANCHOR BOLTS. IF EXISTING ANCHOR BOLTS ARE UNUSABLE CONTACT THE E.O.R. FOR DIRECTION.
6. INSTALL NEW COLUMN AND FIELD WELD TO BEAM ABOVE AS SHOWN. REATTACH KICKERS TO BEAM AND COLUMMN.
7. CONTRACTOR OPTION:
   - SHIP COLUMN BASE PLATE LOOSE.
   - INSTALL BASE PLATE ON ANCHOR BOLTS.
   - INSTALL NEW COLUMN.
   - FIELD WELD NEW COLUMN TO BASE PLATE.

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MAINTENANCE FACILITY
DISTRICT 3 REPAIRS

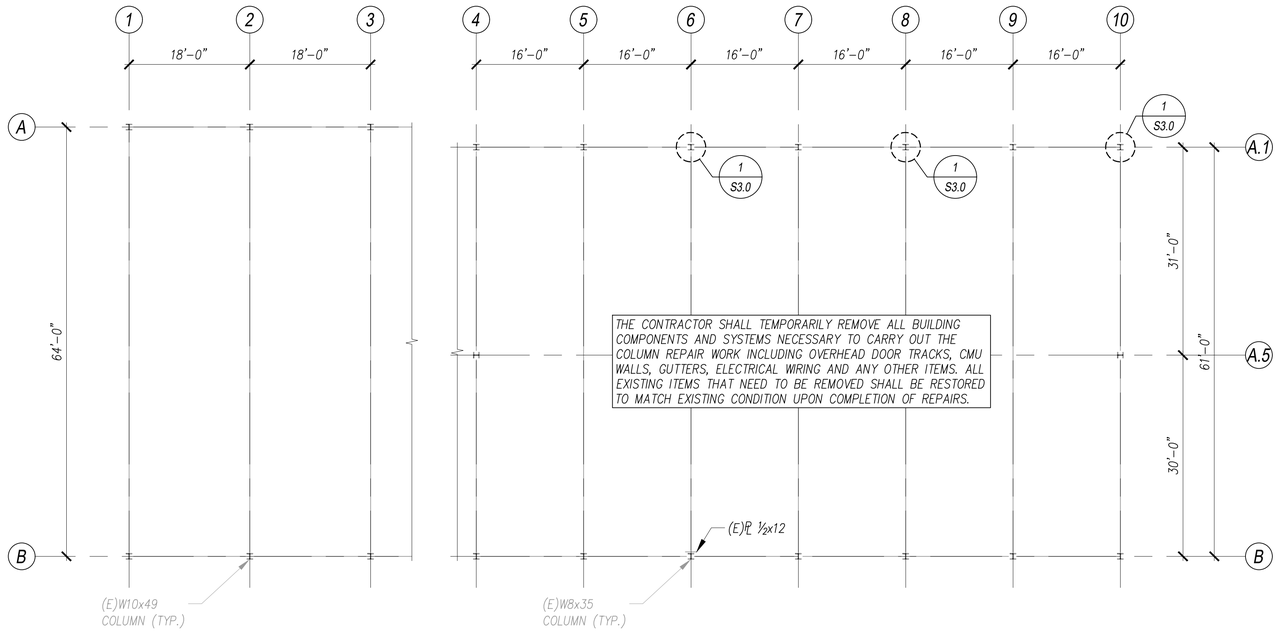
CLAYTON, MO

REVISIONS		
NO.	ISSUE	DATE
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SHEET INFORMATION	
Date:	9/3/2026
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Sheet No.
S2.0

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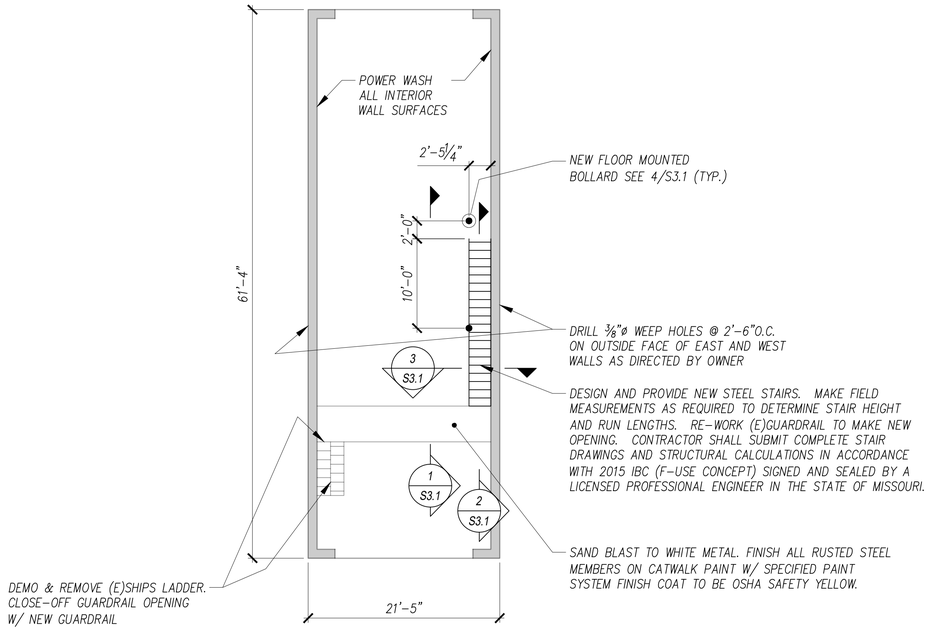


EXISTING MAINTENANCE BUILDING COLUMN PLAN

SCALE: 3/32" = 1'-0"

NOTES:

1. ALL EXTERIOR COLUMNS SHALL BE CLEANED & PAINTED PER THE PAINTING PROCEDURES OUTLINED ON THIS SHEET. ONLY THE BOTTOM 4'-0" OF ALL EXTERIOR COLUMNS SHALL BE PAINTED. ON COLUMNS THAT ARE BEING REPLACED, PAINT SHALL BE APPLIED TO BASE PL PRIOR TO PLACING NEW CONCRETE POUR-BACK.



EXISTING WASH BAY PLAN

SCALE: 1/8" = 1'-0"

PAINTING SYSTEM FOR ST. LOUIS COUNTY MAINTENANCE FACILITIES:

PAINTING SCOPE: ALL SURFACES OF MAINTENANCE BUILDING EXTERIOR COLUMNS (BOTTOM 4' ONLY)

SUBSTRATE: STRUCTURAL STEEL

SYSTEM TYPE: WATER BORNE ACRYLIC SYSTEM

SURFACE PREPARATION (SHOP): ALL NEW STRUCTURAL STEEL SHALL RECEIVE AN SSPC-SP6 COMMERCIAL BLAST CLEANING TO CREATE AN ANGULAR ANCHOR PROFILE OF 1.5 MILS.

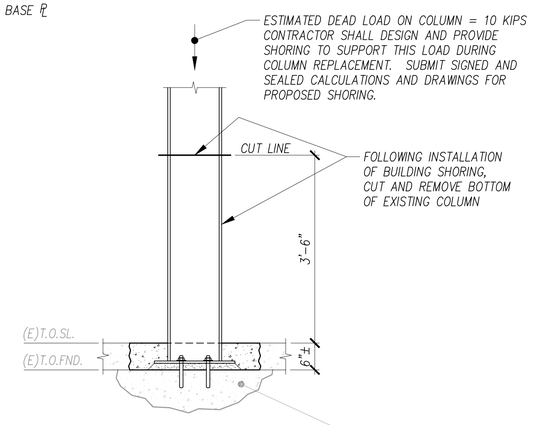
SURFACE PREPARATION (FIELD): ALL EXISTING STRUCTURAL STEEL TO BE PAINTED SHALL RECEIVE A POWER WASH TO REMOVE SALT CONTAMINATION. POWER WASH WITH MINIMUM 3000 PSI WITH OSCILLATING TIP TO REMOVE ALL LOOSE, NON-ADHERENT MATERIAL, CHALK, DIRT, OR ANY OTHER FOREIGN MATTER. SSPC-SP2 HAND TOOL CLEAN OR SSPC-SP3 POWER TOOL CLEAN TO REMOVE ANY RUSTED AREAS. FEATHER SAND RAW EDGES. ALL SURFACES SHALL BE CLEAN, DRY, AND FREE OF CONTAMINANTS.

Approved Painting Systems for Steel

Paint Manufacturer	Coat	Product	Minimum Dry Film Thickness
Pittsburgh Paints (PPG)	Primer Coat	Semi-Gloss 90-712 PITT TECH Acrylic	2 to 3 mils
	Intermediate Coat	Semi-Gloss 7-374 7 Line Acrylic	1.5 to 2.0 mils
	Top Coat	Semi-Gloss 7-374 7 Line Acrylic	1.5 to 2.0 mils
Sherwin Williams	Primer Coat	Semi-Gloss Pro Industrial Pro-Cry Acrylic	1.5 to 2.4 mils
	Intermediate Coat	Semi-Gloss Pro Industrial Multi-Surface Acrylic	1.5 to 2.4 mils
	Top Coat	Semi-Gloss Pro Industrial Multi-Surface Acrylic	1.5 to 2.4 mils

COLOR: MATCH EXISTING AS APPROVED BY OWNER.

NOTE: ABOVE PRODUCTS MAY BE BRUSH, ROLLER, OR SPRAY APPLIED. REFER TO SPECIFIC PRODUCT DATA SHEET FOR ADDITIONAL INFORMATION.

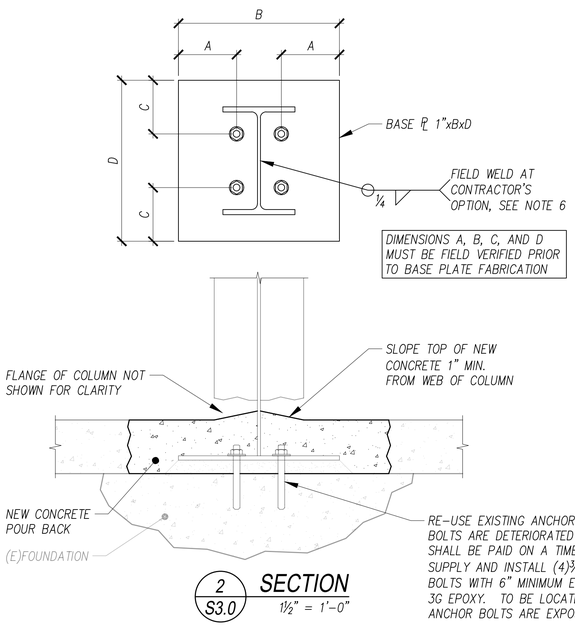


1 S3.0 — ¾" = 1'-0"

DEMO SECTION

NOTES:

1. SHORE BUILDING TO REMOVE LOAD FROM COLUMN, SEE SHORE LOADING ABOVE FOR REQUIRED SHORING STRENGTH.
2. BREAK OUT EXISTING PAVING/CONCRETE SLAB AROUND BASE OF COLUMN TO ALLOW FOR REMOVAL OF EXISTING ANCHOR NUTS, BASE PLATES, GROUT AND THE BOTTOM OF COLUMN.
3. NEATLY SQUARE-CUT EXISTING COLUMN AS SHOWN AND REMOVE BOTTOM OF COLUMN.
4. INSTALL COLUMN BASE PLATE TO MATCH EXISTING. INSTALL NEW COLUMN SECTION TO MATCH EXISTING USING FULL PENETRATION WELD AT CUT LINE. APPLY NEW GROUT BELOW BASE PLATE.
5. RE-POUR NEW CONCRETE AROUND BASE OF COLUMN. SLOPE AWAY FROM COLUMN AS SHOWN IN SECTION 2/S3.0.
6. CONTRACTOR OPTION:
   - SHIP COLUMN BASE PLATE LOOSE.
   - INSTALL BASE PLATE ON ANCHOR BOLTS.
   - INSTALL NEW COLUMN.
   - FIELD WELD NEW COLUMN TO BASE PLATE.



2 S3.0 — 1½" = 1'-0"

SECTION

ISSUED FOR CONSTRUCTION

NOTE: CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL EXISTING & PROPOSED DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

STRUCTURES, INC.
A STRUCTURAL ENGINEERING FIRM
11414 Gravois Rd • Suite 201
St. Louis, MO 63126
314.638.6650

LICENSEE
NAME: Roy Edward Jennings III
DISCIPLINE: Engineering
LICENSE #: E-2010016671
EXPIRATION DATE: 12/31/26

CORPORATION
NAME: Structures, Inc.
DISCIPLINE: Engineering
CERT. OF AUTHORITY #: E-2006035000

STATE OF MISSOURI — ROY EDWARD JENNINGS III — NUMBER E-2020016671 — PROFESSIONAL ENGINEER — 9/3/2026

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ST. LOUIS COUNTY

MAINTENANCE FACILITIES
DISTRICT 3 REPAIRS

CLAYTON, MO

NO.	ISSUE	DATE
7		
6		
5		
4		
3		
2		
1		

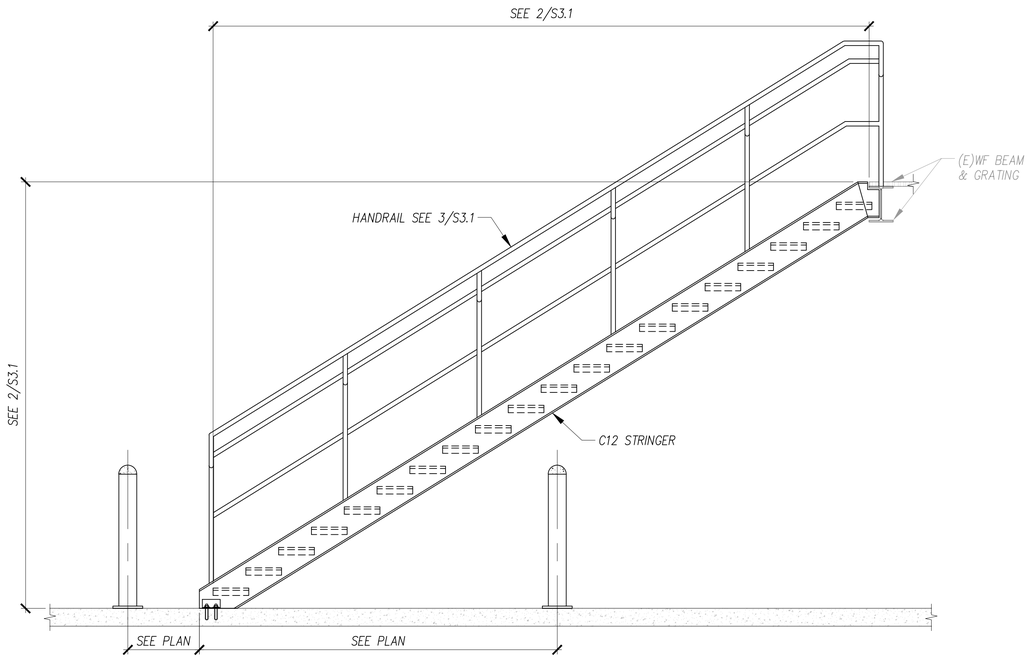
REVISIONS

SHEET INFORMATION

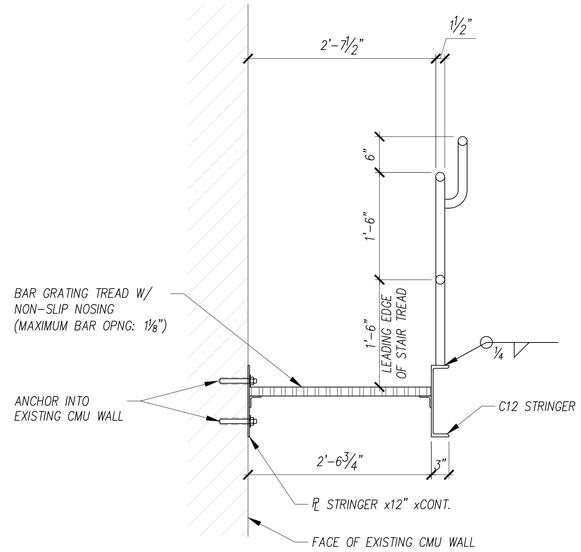
Date:	9/3/2026
Scale:	AS SHOWN
Drawn By:	MPV
Checked By:	REJ
Job Number:	P21-4431-02

Sheet No.
S3.0

TICK MARKS ON THE BINDING MARGIN AND UPPER MARGIN ARE @ 1" CENTERS. IF THE TICK MARKS ARE NOT @ 1" CENTERS, THIS SHEET HAS BEEN REPRODUCED AT A SCALE OTHER THAN 1:1
PRINT TO SCALE SHEET SIZE: 24x36 9/3/2026 9:44 AM

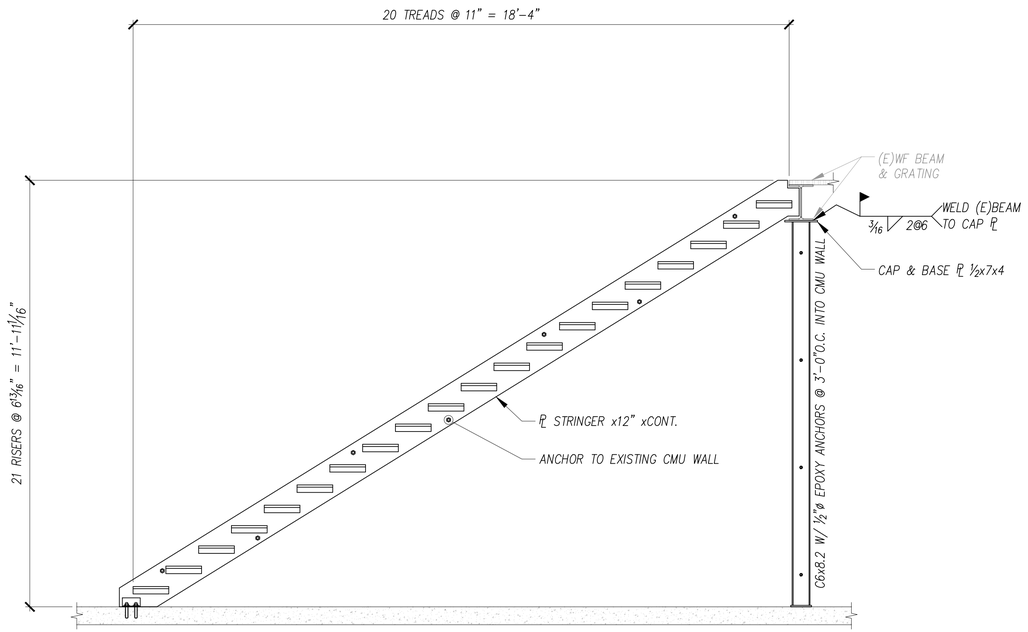


1 S3.1 **SECTION**
½" = 1'-0"

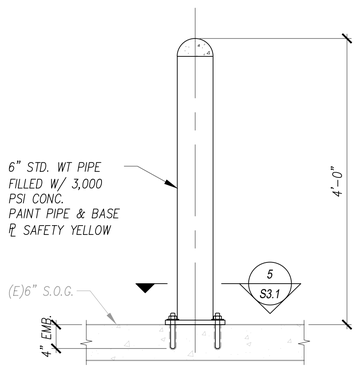


3 S3.1 **STAIR SECTION**
1" = 1'-0"

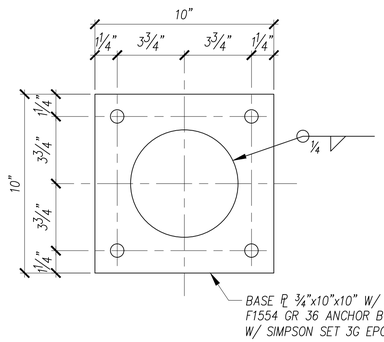
NOTES:
1. ALL STEEL STAIR FRAMING, TREADS, RAILS, & BOLTS TO BE HOT DIPPED GALV. PER ASTM A123 OR ASTM A153, AS APPLICABLE, & PAINTED OSHA SAFETY YELLOW.



2 S3.1 **STAIR SECTION**
½" = 1'-0"



4 S3.1 **SECTION**
1" = 1'-0"



5 S3.1 **SECTION**
3" = 1'-0"

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STATE OF MISSOURI · ROY EDWARD JENNINGS III · NUMBER E-2020016671 · PROFESSIONAL ENGINEER · 9/3/2026

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ST. LOUIS COUNTY
MAINTENANCE FACILITY
DISTRICT 3 REPAIRS
CLAYTON, MO

NO.	ISSUE	DATE
7		
6		
5		
4		
3		
2		
1		

REVISIONS

SHEET INFORMATION

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